

Application of Reservoir Performance Indices on Kainji Reservoir System

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Abstract

Resilience may be defined as a yardstick that specifies the extent a reservoir recuperates when it fails previously. A reliable water resource system speedily returns to an acceptable state after a failure. Vulnerability measures severity or extent of failures or letdowns, if and once they occur. Sustainability index (SI) provides a sign of fundamental nature with respects to probable unwanted repercussions if imbalance of waters occurs. Sustainability index (SI) can also be expressed as a mean of reliability, resilience and vulnerability. This study employs commonly used indices (reliability, resilience, and Vulnerability) to assess the performance of Kainji reservoir system. To attain this, rain fall and river flow data were obtained from Kainji Hydrological station in New Bussa Nigeria. Analysis using MAKSESENS software was used on the rainfall and river flows to look at the extreme events. In order to determine the performance of the reservoir system some reservoir performance indices were employed, these are; vulnerability, resilience, and sustainability index. This is achieved by adopting relevant existing equations. Reservoir flows and storages were employed, different draft ratios were considered (0.3- 1.0). Varying demand levels were also chosen (0 – 0.8) as against the coefficient of variation to look at the monotonic behaviour of resilience against the various levels of demand. Volume reliability falls repeatedly as draft/ MAR upsurges and bigger values were attained as S/MAR rises. The analysis on the reservoir shows that as demand decreases the sustainability increases and also the higher the storage ratio the higher the sustainability index. It also shows that as draft ratio increases the resilience decreases, and because the draft ratio decreases the resilience increases. The reservoir system was classified as within year system, which suggests high resilience, less vulnerable and sustainable. The operation rule shows the need for optimization.

Keywords: reservoir, operation, inflow, outflow, storages randomness

1. Introduction

Reservoirs are hydraulic structures that play central role in the growth of any nation. It's the main costly element within the multiuse river basin in any geographical region. They require very cautious planning, design, construction, and operation (Yafang *et al.*, 2019). A reservoir could be an artificial water catchment or big freshwater body which is employed for numerous purpose activities for example; hydropower generation, water system, and regulate flooding, to safeguard the environment. The challenges of the water system as a result of the probability factors like randomness of rainfall and runoff make the management of the reservoir system a difficult one.

The key reservoir measuring instruments are broadly applied tools to assess the effect of water resource systems on a lifelong position. In putting to use a water resource storing system, letdown is assumed to have happen when request at a precise period say four weeks surpasses water supply. As we change to the succeeding week's period, water storage mechanism may stay within letdown situation or can shift to attainment condition. Within a latter situation, present reservoir letdown is assumed overtaken. The reservoir letdown incident is categorized into two facets: amount of four weeks within which a specific incident occurs and plus the deficit.

Failures inside the operation of a reservoir have numerous facets: quantity, degree, severity. The aforementioned water resource measuring indicators are accustomed to quantify dissimilar parts as related to the effect of a reservoir mechanism. Typically the calculations of these indicators are done on monthly or yearly obtained figures (data). (Sharad, 2010).

1.1 Reliability

largely water storage system rule operation are trailed in twofold indices. It is important to note that time, otherwise incidence centred on dependability of a water storage system can be described as the likelihood that a reservoir system state is within an acceptable positon.

$$r_t = 1 - \left(\frac{f_p}{n} \right); 0 \leq r_t \leq 1, F_p \leq n \quad (1.1)$$

where r_t = periodic dependability while F_p = amount of letdowns times in all of the N times. r_v = is given as Volume or quantity-based reliability

$$r_v = V_s/V_d \quad (1.2)$$

Where V_s = the quantity of supplied water and V_d = quantity of water needed during a given time. Giving N as the all amount of letdown occasions. Stirring from periodic stage t toward $(t + 1)$, a reservoir shall whichever continue to be in the similar position or move to the other positon. A period of the j^{th} letdown events represented by d_j plus v_j is the conforming shortfall size determined by:

$$EV_j = \sum_{t=1}^{d_j} [D_t - R_t] \quad (1.3)$$

Where D_t = target demand and R_t = release from the reservoir for the month t .

1.2 Resilience

Resilience(c) defines speed to which a water storage system (reservoir) is possibly going to recuperate after let down. It's the same as the mean likelihood of regaining after letdown of a distinctive solitary time phase and can this is equated to the opposite of the average time which the system uses when at undesirable condition:

$$\gamma_{mean} = \frac{1}{N} \sum_{j=1}^N d_j \quad (1.4)$$

Moy. *et al.*, (1986) explained resilience in the context of highest sequential time period of system lasting in deficient poor suboptimal state. Whereas as reported in Kjeldsen and Rosbjerg (2004), resilience is the opposite of the utmost failure duration. This is often explained according as in equation (1.5).

$$\gamma_{max} = \frac{\max\{d_j\}}{j} \quad (1.5)$$

where γ_{max} = resilience, d_j = number of failure states.

Relating to his research, Kundzewicz and Kindler (1995), explained that the quantification of resilience grounded on extreme values are more preferred instead of the average figure quantification for the reason that insignificant inconsequential events may bring down the average value. Kjeldseen and Rosbjarg (2004) juxtaposed the twofold guesstimates of resilience together through approximation by means of 0.90th fractile for experimental Cumulative Distribution Function (CDF) of let-down period plus the shortfall capacity. They also promoted the application of extended chains of simulated data to get strong and dependable approximations.

According to Hazen, followed by Sodler, and Hurst (1985), presented one among the foremost beneficial indices of reservoir system performance, here defined the resiliency index as:

$$m = \frac{(1-\alpha)\mu}{\sigma} = \frac{1-\alpha}{Cv} \quad (1.5)$$

where α = annual yield as a segment of the average yearly inflow μ , σ = discrepancy of the yearly inflows, and Cv = coefficient of variation of the yearly streamflow.

Figure 1.1 described the interactions among m , σ and Cv given in equation (1.10) and in line with Vogel (1996), Perrens and Howell (1972), the term which is the standardized inflow was also used by Hurst, after which the non-dimensional index m has subsequently found use in both analytical investigations in "water storage theory" and in town findings of the storage-reliability-yield relationship. Vogel & Stedinger (1987) recommended that so far $0 \leq m \leq 1$, the system is overcomed by over-year behaviour, σ while if $m > 1$, the reservoir can be said to be

dominated by within-year behaviour. Systems that are having small resiliency (m near zero) can be said to be categorized to be of either larger values of Coefficient of variation or as yearly yield or both (fig. 1.1). in the same vain, water recourse systems that have values of m close or beyond unity are very probable to fill-up once empty and therefore those kind of systems are more expected to show within-year in its place of over-year behaviour.

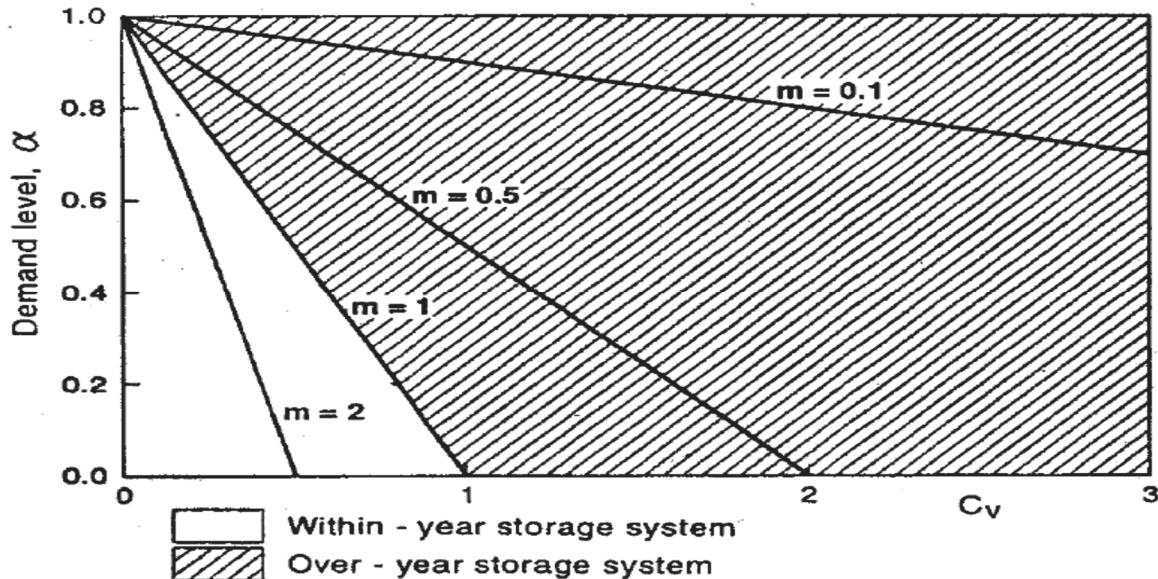


Figure 1.1 The demand level α as a function of the resiliency index m and the Coefficient of variation of the inflows Cv . (Richard, 1996)

1.3 Vulnerability

Vulnerability can be said to be a way of quantifying the impairment done in a failure event.

Kjeldsen and Rosbjerg (2004), projected vulnerability in the context of the average of the shortfall events V_j as:

$$V_{mean} = \frac{1}{N} \sum_{j=1}^N V_j \quad (1.6)$$

But Kundzewicz and Kindler (1995) suggested that the employment of a maximum event might yield a stronger estimate of vulnerability. To these effect, vulnerability may be computed as in equation (1.7).

$$V_{max} = \max_j \{v_j\} \quad (1.7)$$

Usually, V_{max} is recorded in volume standard units. Mc-Mahon *et al.*, (2006) employed non-dimensional vulnerability ratio by fractioning V_{max} and objective demand. Hence, the existence of diverse methods to quantify resilience and vulnerability, reliant onto if the average or the utmost numeric magnitude of the variable signifying failure is accepted.

1.4 Sustainability index (SI)

Recently, some efforts were put up in order to measure the magnitude of how sustainable a reservoir will by adopting Resilience (R), Reliability (R) and Vulnerability (V). Zongxue *et al.*, (1998), postulated a guide designated as drought risk index (DRI), which include the RRV:

$$DRI = \beta_1(1 - r_t) + \beta_2(1 - \gamma) + \beta_3(1 - V) \quad (1.8)$$

where $\beta_1 + \beta_2 + \beta_3 = 1$. No procedures are accessible to select β weights, however Loucks (1997) suggested an equation model termed sustainability index (k) according as

$$K = r_t \gamma (1 - V_{max}/D) \quad (1.9)$$

where D is termed draft, V_{max} = vulnerability maximum, V = vulnerability.

Performance indices i.e. reliability, resilience, and vulnerability have been emphasised in several studies such as Moy *et al.*, (1986), Kundzewicz and Laski (1995), Vogel and Bolognese (1995), Kundzaewicz and Kindller (1995),

Thomas (2005) and Mc-Mahon *et al.*, (2006). Zorica and Bojan (2017), Kang *et al.* (2019), Sharad (2010) and (Hui, *et al* 2019). The objective of this study was to adopt these indices for kainji reservoir system evaluation.

2. Materials and Methods

2.1 Materials

The hydrological data employed for this study were stream flow, rainfall and reservoir level.

2.1.1 The Study Area

By location, Kainji hydroelectric dam is situated in New Bussa in Niger State, Nigeria. The river is made behind the dam width between latitude $9^{\circ} 8'$ to $10^{\circ} 7'$ and between longitude $4^{\circ} 5'$ to $4^{\circ} 7'$ E. (Dukiya,2013). The mean yearly rainfall is 2200mm. Two types of rivers are identified, the black and white. The first drives its tributaries outside Nigeria peaking at about $2,000\text{m}^3/\text{sec}$ in February (Oyebande *et al.*, 1992) and the second drive its source from local tributaries peaking at 4,000 to $6,000\text{m}^3/\text{sec}$ at September to October.

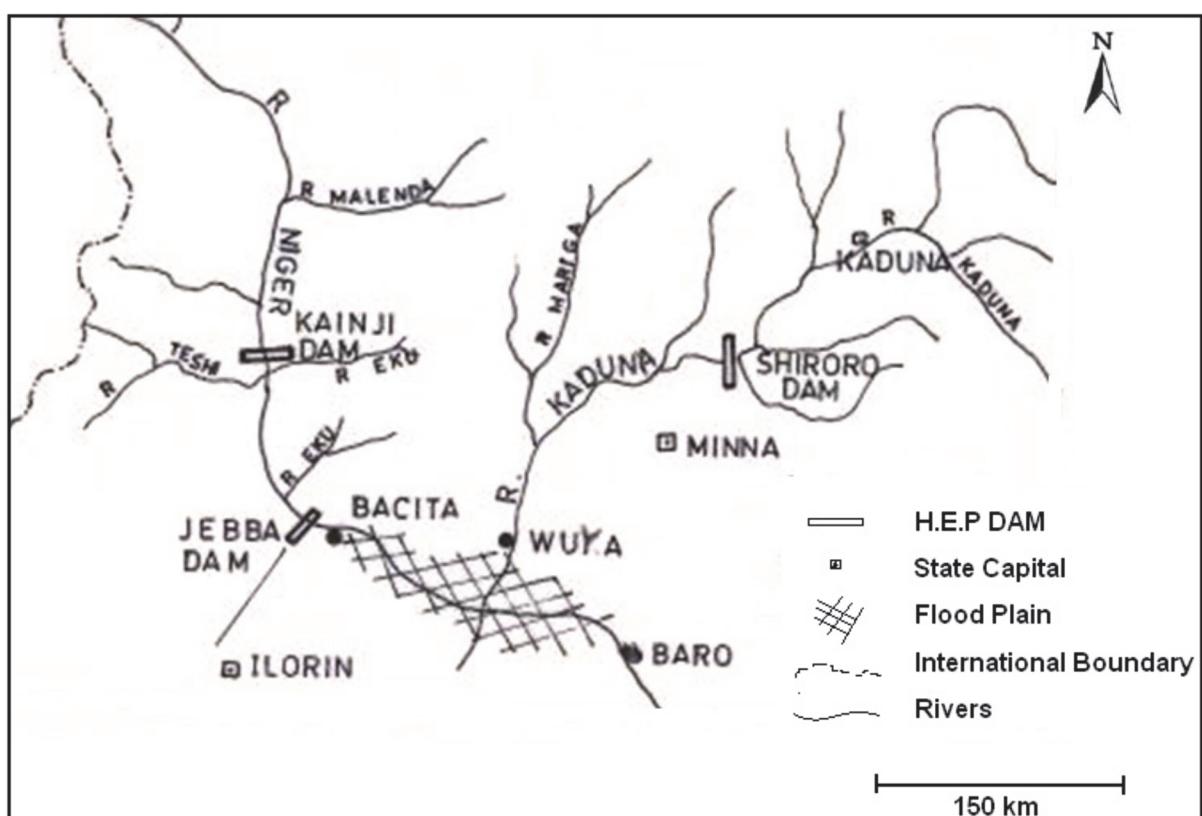


Figure 2.1. Position of Kainji Hydroelectric Dam.

Source: Salami: (2013)

2.1.3 Source of Data

This research is made of fluctuating time-based hydro – meteorological time sequence. It's constituted from monthly inflow and reservoir draw- down data for period of 25 (1990 to 2014) and 20 (1995 to 2014) years, correspondingly, monthly mean rainfall covering 50 (1965 to 2014) and 45 (1969 to 2013) monthly outflow were also acquired from the Hydrology section of Kainji dam, Niger state. A mean daily flow of $2280\text{m}^3/\text{sec}$ is demanded to sustain full generating capacity of 760MW (Alabaide, 2003).

3. Methods

3.1 Analysis of Reservoir Performance Indices

(i) Trend Analysis

MAKESENS was used in estimating trends time series of the mean yearly and monthly (seasonal Mann-Kendall) rain fall, the reservoir level or draw-down and therefore the river flows. The obtained data sequences were divided into wet and dry period of year and twelve calendar months. The process is predicated on the nonparametric Mann-Kendall test for the trend and therefore the nonparametric Sen's technique for the extent of the variation.

(ii) Volume Reliability Index

The volume reliability or quantity based reliability R_v was estimated employing the equation below:

$$R_v = V_s/V_d \quad (1.12)$$

Where V_s is the volume of water supplied and V_d is the volume of water demanded during a given period.

In determining volume reliability as a function of draft/MAF, different values of draft ratios were considered i.e., (0.3 – 1), as in the works of Sharad (2010), this was to view the monotonic behaviour of volume reliability against draft/MAF.

(iii) Sustainability Index

The sustainability index was computed using the equation below:

$$k = \frac{r_t(1-V_{max})}{D} \left(\frac{S_{max}}{MAF}\right) \quad (1.13)$$

where r_t is the time reliability, V_{max} is max Vulnerability, D is draft, S_{max} is maximum storage, MAF is mean annual flow.

Different values of draft ratios were considered i.e., (0.3 – 1), as in the works of Sharad (2010), this was to view the monotonic behaviour of sustainability index against draft/MAF.

(iv) Resilience Index

The resilience index which describes how quickly a system is probably going to pass through failure was evaluated employing the relationship:

$$m = \frac{1-\alpha}{C_V} \quad (1.14)$$

Where m is the resilience, α is the demand level, and C_V is the coefficient of variation. Different demand levels were chosen (0 – 0.8) as against the coefficient of variation to view the monotonic behaviour of resilience against the different levels of demand.

(v) Vulnerability Index

Vulnerability which measures the likely magnitude of a failure was estimated using the equation below:

$$V_{mean} = \frac{1}{N} \sum_{j=1}^N V_j \quad (1.15)$$

where v_j is the mean value of the deficit events, N is the period

3.2 Classification of Reservoir System

The reservoir system Characterization was carried out by using equation (1.16) as suggested in (Mohammed *et al.*, 2018).

$$m = ((1-\alpha))/C_V \quad (1.16)$$

where and m is referred to as the resiliency index, CV the coefficient of variation, whereas α is the demand. The resiliency index (m) was calculated by taking the ratio of demand subtracted from one and also the coefficient of variation (CV); similarly CV was computed by taking the ratio of the standard deviation to the averages of the inflows. On the opposite hand, the demand levels were hypothetically fixed at 0, 0.2, 0.4, 0.6 and 0.8 in line with reservoir limit conditions (i.e. if $0 \leq m \leq 1$, the reservoir system is dominated by over year behaviour, whereas if $m > 1$) the system is dominated by within-year behaviour) as recommended within the works of Vogel and

Stedinger, (1987), Sharad (2010), and Issa et al., (2014). On the premise of this, two reservoir states were identified, (1) within - year storage (2) over – year storage.

4. Result and Discussion

4.1 Reservoir Flow and Storages

In describing the system flow and storages, Figure 3.1 presents monthly inflow storage and demand levels of kainji reservoir system. The smallest amount inflow was within the month of April and highest within the month of October. The month of October have peak outflow and storages. While maximum demand was within the month of September. The magnitude of the values obtained in September and October might be as results of peak rainfall within the time frame as might be observed in figure 3.6.

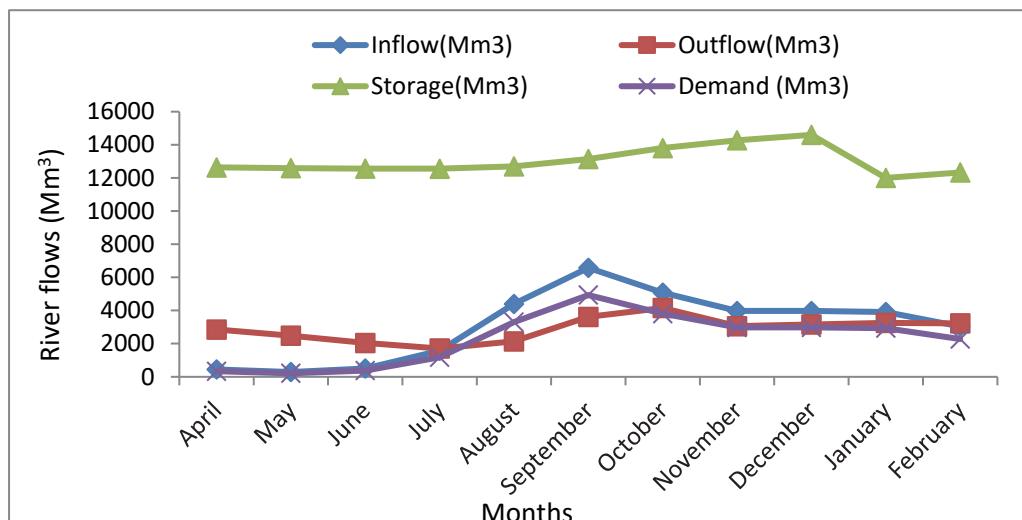


Figure 3.1. Monthly Inflows, Outflows, Storage and Demand Regime

Figure 3.2 illustrates the comparative disparity and dispersion within the averages of the rainfall respectively; it indicates the occurrence of seasonal effects within the moments, connoting that monthly statistics for dry season are considerably different comparatively from the raining season time regime. Distinct from irregular stream flow processes, the seasonal averages have greater values than the seasonal deviations all the years under consideration. The variance is maximum during lately rains and emerging dry season; more or less the interfacing period. This means atmospheric volatility during this watershed period; i.e., the fringes of the raining season moving into full Harmattan period.

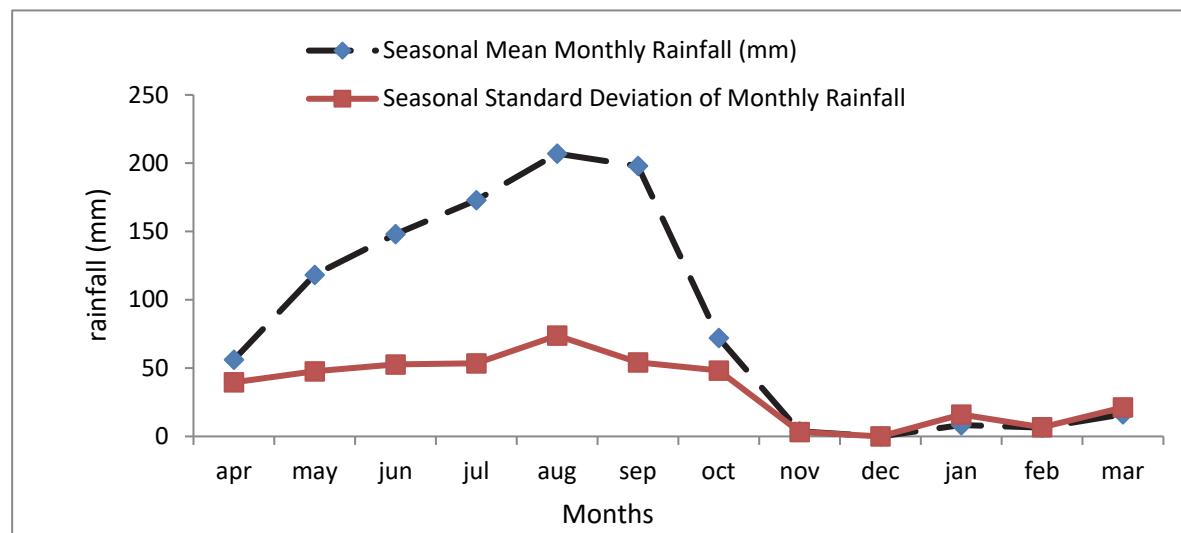


Figure 3.2. Variation in Seasonal Moments

3.2 Trend Analysis

Figure 3.3 shows the inter- annual decadal variation within the rainfall series; long term pattern is apparently evident. However, there's large changeability among the monthly values of rainfall of various years, with the period 1985 – 1994 showing minor increases within the storm manifest during peak seasons.

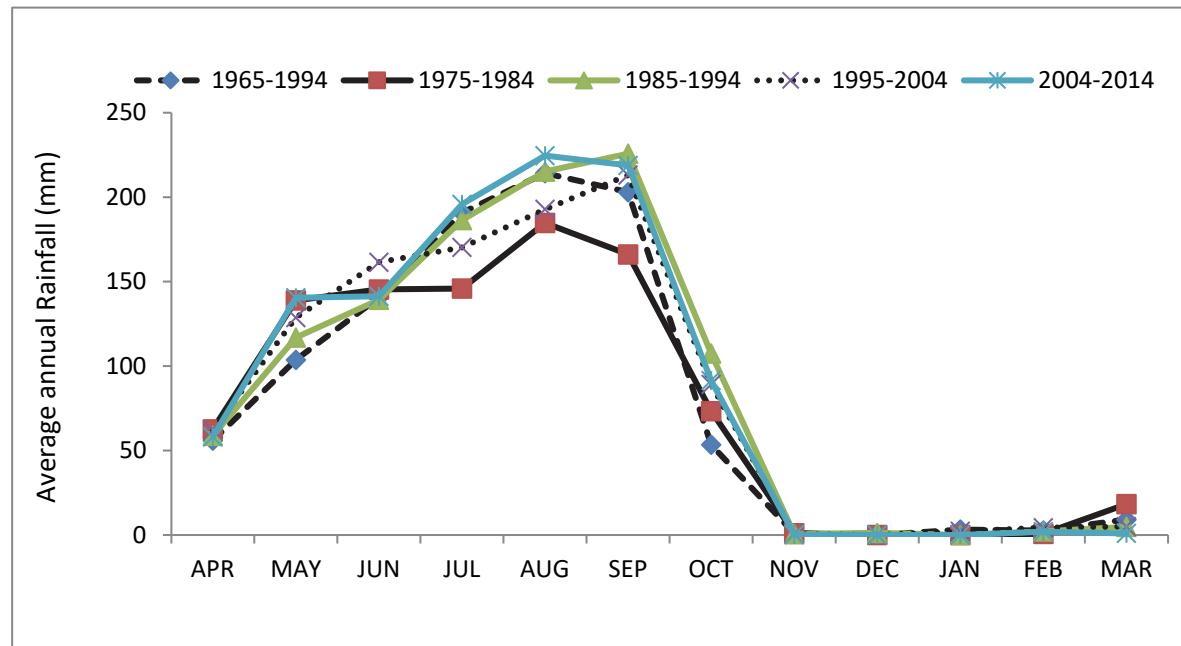


Figure 3.3. Inter- annual Variations in rainfall

The time - period of 1975 – 1984 shows all-time low rainfall decadal period. The rainstorm upsurges from the early raining period of April (hydrological period) of the year to the height period of September. The months of November to March shows dry periods.

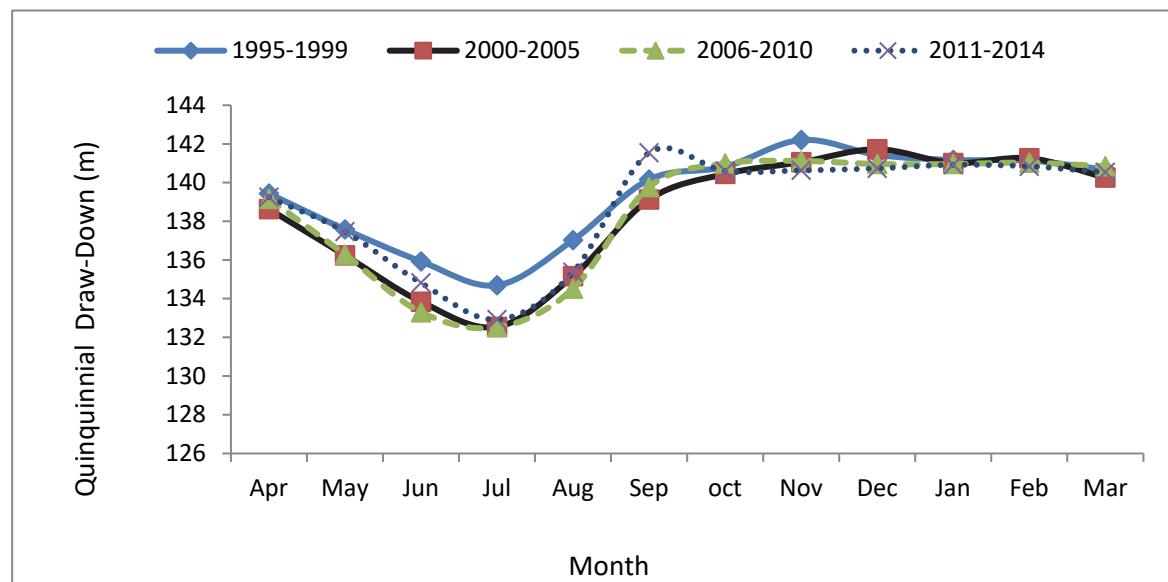


Figure 3.4. Trend of Average Monthly Draw – Down

Figure 3.4 and 3.5 display the five yearly intervals and ten yearly intervals mean annual draw- down of the reservoir. The lowermost reservoir level of 133.134m was in the month of July and December getting the maximum reservoir level of 141.17m throughout the time frame considered. The minimum reservoir level in July can be attributed to

initial period of rainfall and therefore the maximum at the month of December can be accredited to extreme period of rainfall in August plus September.

Figure 3.5 describes the inter- annual decadal variation within the reservoir level; long term pattern is apparently evident. Also there's large inconsistency among the monthly values of reservoir level of various years, with the time period of 1995 – 2004 showing slight increases within the draw-down with peak of 141.56m, evident during peak seasons. The water level of the reservoir is at very low level within the month of July with reservoir level at 132.66m in 2005- 2014 time period. Figure 3.4 describe obviously seasonal or periodic pattern; it is a periodic-stochastic series. The analysis of the step or jump trend of the reservoir draw – down indicates that the critical value $t_{0.05}(18)=1.734$, $t = 4.46$ in order that the hypothesis H1 is accepted, as $t(\mu_1 \neq \mu_2)$, meaning that the step or jump trend is significant at 5% probability.

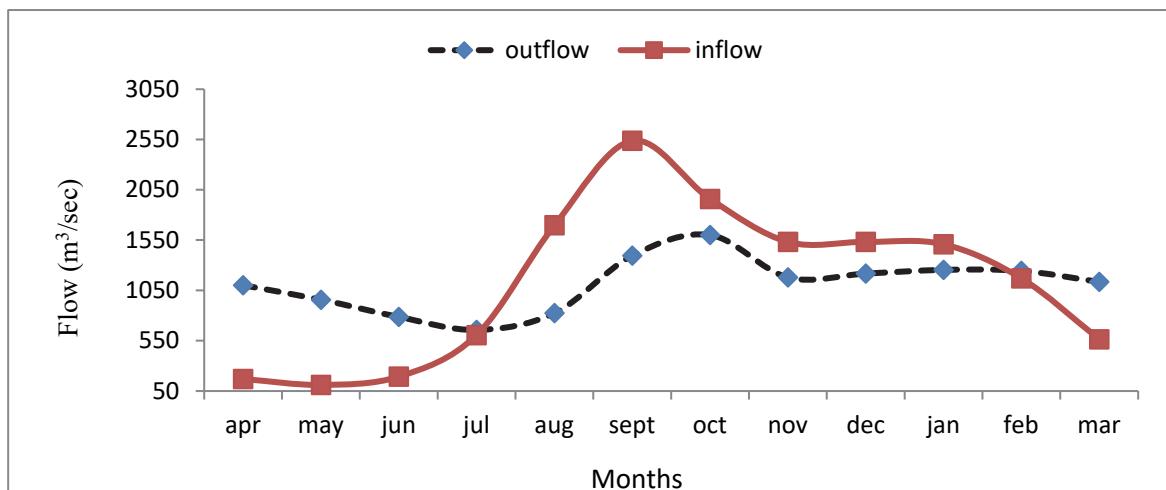


Figure 3.6. Trend of Monthly Flow of Kainji

The monthly reservoir flows i.e., inflow and outflow, is displayed in figure 3.6. The peak inflow was within the month of September with a maximum of 2516.39m³/sec during the time frame of peak storm. Similarly the outflow was at the highest within the month of October with a recorded value of 1456.98m³/sec. The low flows recorded were within the periods of early rainstorm.

3.3 Reservoir Operating Policy

Figure 3.8. Shows operation rule of kainji reservoir system, it describes level of storages as it relates to months. The lowest storage was in the month of January with 12000Mm³ and peaking at 14602.58Mm³ in December. The peak period in January could be attributed to heavy rainfall in the months of August, September and October as the flood which finds its way to the river far away from the dam in Nigeria. The reservoir could be said to be reliable looking at the storage capacity through the months but there is need to increase storages and reduce excessive demands by optimizing the operation policy. The gap between the antecedent operation policy and SLOP operation policy implies the need for increases in storages so as the increase the reliability of the reservoir.

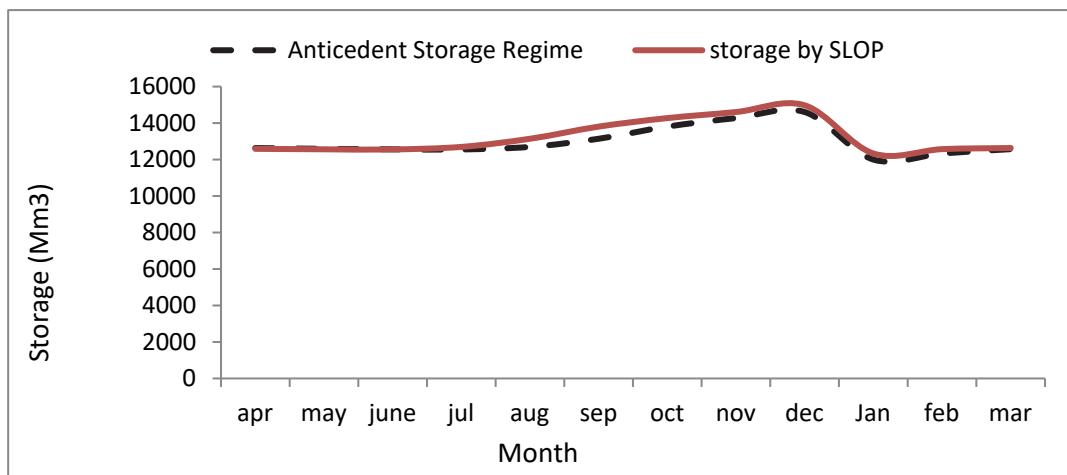


Figure 3.8. Kainji reservoir storage rule curve

3.4 Reservoir Performance Indices

(i) Volume reliability index

In order to establish the performance of the reservoir system some reservoir performance indices were employed, these are; vulnerability, resilience, and sustainability index. The results are as presented below.

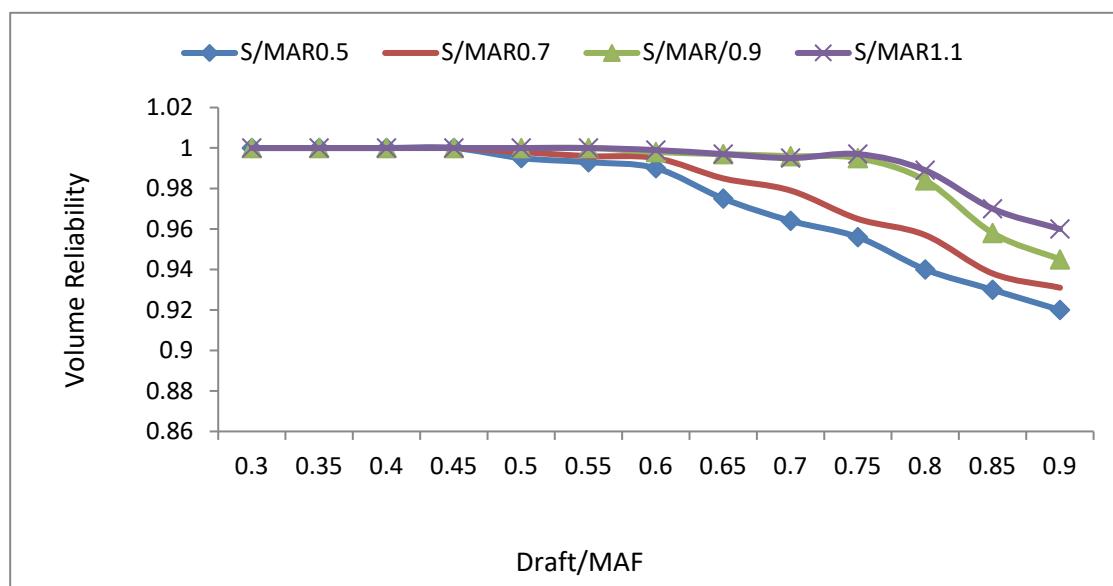


Figure 3.9. Variation of volume reliability with draft ratio

Figure 3.9 expresses volume reliability as a function of draft/MAR for diverse values of storage ratios. In this case volume reliability falls repeatedly as draft/ MAR upsurges and bigger values are gotten as S/MAR grows. It imperative to observe that the gap that exist between the curve for storage fractions of 0.7 and 0.9 are more juxtaposed to that between 0.5 and 0.7, etc. The implication of this is that the larger storage is provided, then improvements in reliability are increasingly smaller. This agrees with the reservoir SLOP recalibrated policy of need to increase storage for better performance. This also means that the antecedent operating policy is less effective owing to its lower storage levels and consequently leading to higher vulnerability (0.072) especially at higher demand levels.

(ii) Sustainability Index

Figure 3.10 depict the relationships between sustainability index and draft as a ratio of the mean annual runoff.

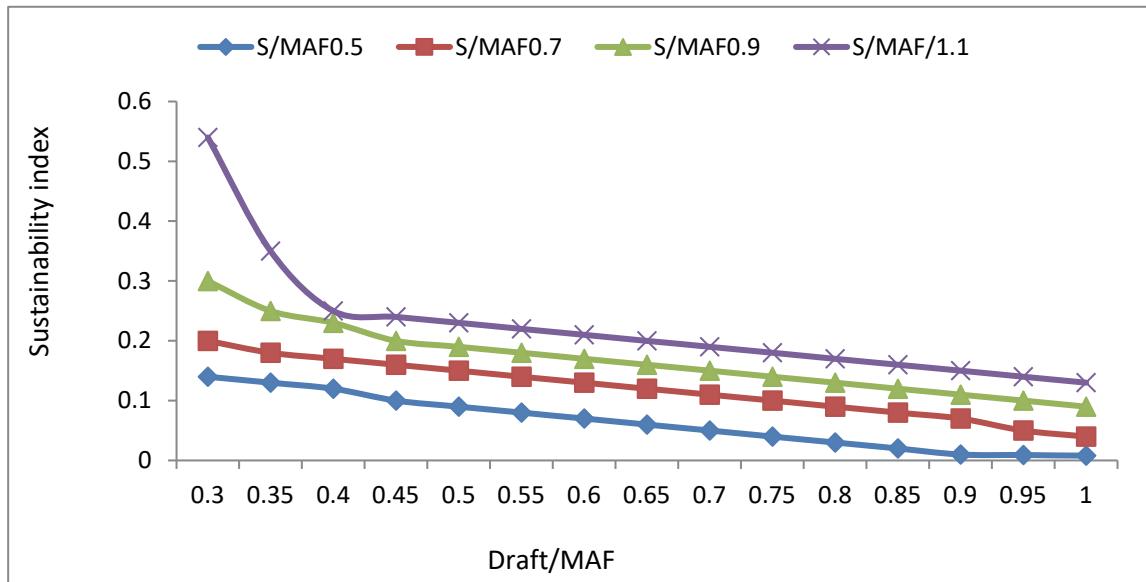


Figure 3.10. Variation of sustainability index with draft ratio

The figure 3.10 shows that as demand decreases the sustainability increases and the higher the storage ratio the higher the sustainability index. The figure shows that sustainability index has a monotonic variation with draft ratio. Figure 3.10 substantiate the recalibrated SLOP policy indicating the need for increase in storage ratio during months of incipient rainfall and low flow and during periods of high flow in preparation for the dry periods. This also indicates that the antecedent reservoir operating policy is less sustainable (0.54) owing to its lower storage levels.

(iii) Resilience

Resilience (m) describes how quickly a system is likely to recover from failure. Figure 3.11: shows the variation of resilience with draft ratio.

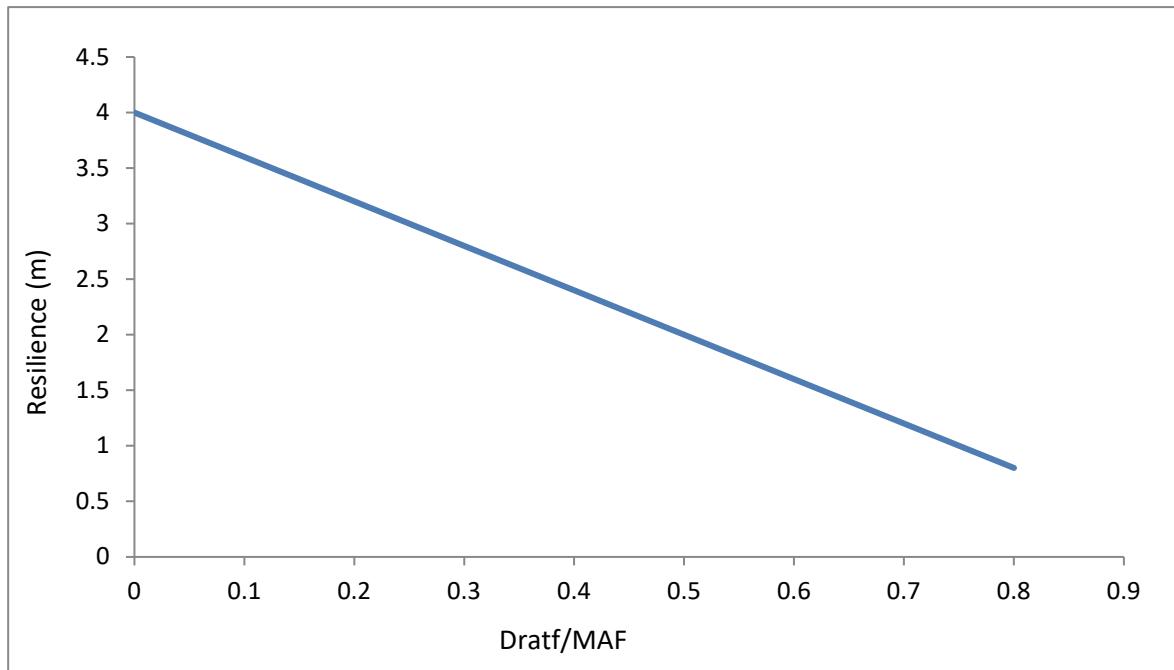


Figure 3.11. Variation of resilience with draft ratio

Figure 3.11 depicts the relationship between the resilience index and the draft ratio, it shows that as draft ratio increases the resilience decreases, and as the draft ratio decreases the resilience increases. This is imperative relating to the SLOP recalibrated reservoir policy depicting increase in the storage ratio, increase in storage ratio increase the reliability of reservoir system this substantiate the output of the recalibrated policy. The computed parameters of reservoir indices are presented in table 3.2 below.

Table 3.2 Reservoir Performance Indices

Index	Draft Ratio	Antecedent reservoir Parameter Values (%)	policy Index	SLOP recalibrated Parameter Values (%)	Index
Reliability	0.75	0.92		0.96	
Sustainability	0.75	0.54		0.76	
Vulnerability	0.75	0.072		0.036	
Resilience	0.75	1		1	
Volume Reliability	0.75	0.95		0.99	

3.5 Classification of Reservoir System

The result of the reservoir classification or characterisation is as presented in Table 3.3 below.

Table 3.3 Values of Resilience (m), Demand level (α), and Coefficient of Variation**(CV) Obtained to Characterise the Reservoir System**

Demand Level (α)	Coefficient of variation (CV)	Resiliency(m)	Characterization
0	0.25	4	Within year
0.2	0.25	3.2	Within year
0.4	0.25	2.4	Within year
0.6	0.25	1.6	Within year
0.8	0.25	0.8	Over year

Table 3.3 shows the characterization of the reservoir system, the reservoir system is characterized as within year system considering the reservoir characterization limit condition that if ($m > 1$) and the coefficient of variation is low, the reservoir is within year system. The resilience indices m increases with reduction in the demand level which depicts that the higher the demand level the less the reservoir resilience.

4. Conclusion

From the study it could be concluded that;

1. The extreme events of the rail fall are between September and January while the reservoir flows are between May and September.
2. The reservoir is less vulnerable, resilient and sustainable
3. The classification of the reservoir system as within year system is an indication of resiliency, and sustainability.
4. The higher the demand level the more vulnerable, less resilient, and less sustainable the reservoir hence the need for optimization of the operation rule.

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